

Notes

- ◆ Make-up lecture tomorrow 1-2, room 204

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Linear basis in 1D

- ◆ From last-time, the equation for test function i was:

$$\frac{1}{x_{i+1}-x_i}u_{i+1} - \left(\frac{1}{x_{i+1}-x_i} + \frac{1}{x_i-x_{i-1}}\right)u_i + \frac{1}{x_i-x_{i-1}}u_{i-1} = \int f(x)\phi_i(x)dx$$

- ◆ Can match up left-hand-side (matrix) to finite difference approximation
- ◆ Right-hand-side is a bit different:

$$f(x_i) \quad \text{vs.} \quad \int f(x)\phi_i(x)dx$$

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The Mass Matrix

- ◆ Assuming f is from the same space, get:

$$\int f\phi_i = \int \sum_{j=1}^n f_j\phi_j \phi_i = \sum_{j=1}^n \left(\int \phi_i\phi_j\right) f_j = (Mf)_i$$

- ◆ M is called the **mass matrix**
 - Obviously symmetric, positive definite
- ◆ In piecewise linear element case, tridiagonal

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Lumped Mass Matrix

- ◆ The fact that M is not diagonal can be inconvenient
 - E.g. if solving a time-dependent PDE, M multiplies the time derivative - so even an “explicit” method requires solving linear systems
 - Can be viewed as a low-pass / smoothing filter of the data, which may not be desired
- ◆ Thus often people will “lump” the offdiagonal entries onto the diagonal: **lumped mass matrix** (versus “consistent” mass matrix)
- ◆ This makes the connection with finite differences (for piecewise linear elements) perfect

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Stiffness matrix

- ◆ The matrix A (where derivatives show up) is called the **stiffness matrix**
 - “Stiffness” and “mass” come from original FEM application, simulating solid mechanics

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Assembling matrices

- ◆ Entry of the stiffness matrix:

$$A_{ij} = \int_{\Omega} \nabla\phi_i \cdot \nabla\phi_j = \sum_e \int_e \nabla\phi_i \cdot \nabla\phi_j$$

- ◆ Here we sum over “elements” e where basis functions i and j are nonzero
 - Usually an “element” is a chunk of the mesh, e.g. a triangle
- ◆ Can loop over elements, adding contribution to A for each
 - Each contribution is a small submatrix: the **local** (or **element**) stiffness matrix
 - A is the **global** stiffness matrix
 - Process is called **assembly**

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Quadrature

- ◆ Integrals may be done analytically for simple elements
 - E.g. piecewise linear
- ◆ But in general it's fairly daunting – or impossible (e.g. curved elements)
- ◆ Can tolerate some small error: numerically estimate integrals = **quadrature**
- ◆ Basic idea: sample integrand at **quadrature points**, use a weighted sum
 - Accuracy: make sure it's exact for polynomials up to a certain degree

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FEM convergence

- ◆ Let the exact solution be u^*
and for a given finite element space V let the numerical solution be $u \in V$

- ◆ Galerkin FEM for Poisson is equivalent to:

$$u = \arg \min_{u \in V} \int_{\Omega} \|\nabla(u - u^*)\|^2$$

(closest in a least-squares, semi-norm way)

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FEM convergence cont'd

- ◆ Don't usually care about this semi-norm: want to know error in a regular norm. With some work, can show equivalence...
- ◆ The theory eventually concludes: for a well-posed problem, accuracy of FEM determined by how close function space V can approximate solution
- ◆ If e.g. solution is smooth, can approximate well with piecewise polynomials...

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Some more element types

- ◆ Polynomials on triangles etc.
- ◆ Polynomials on squares etc.
- ◆ More exotic:
 - Add gradients to data
 - "Non-conforming" elements
 - Singularity-matching elements
 - Mesh-free elements

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Mesh generation

- ◆ Still left with problem: how to generate the underlying mesh (for usual elements)
- ◆ More or less solved in 2D, still heavily researched in 3D
- ◆ Triangles/tetrahedra much easier than quads/hexahedra
- ◆ We'll look at one particular class of methods for producing triangle meshes: **Delaunay triangulation**

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Meshing goals

- ◆ Robust: doesn't fail on reasonable geometry
- ◆ Efficient: as few triangles as possible
 - Easy to refine later if needed
- ◆ High quality: triangles should be "well-shaped"
 - Extreme triangles make for poor performance of FEM - particularly large obtuse angles

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